ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. THE PUBLICATIONS LISTED BELOW ARE THE GOVERNING CODES AND STANDARDS AND REFERENCED BY THE BASIC DESIGNATION. IN THE CASE OF CONFLICTING APPLICABLE CODES AND STANDARDS:

## BUILDING CODE BOCA NATIONAL BUILDING CODE, 1996 EDITION

- AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," JUNE 1, 1989. AISC
- AISC SPEC. FOR AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM STRUCT. JOINTS A325 A490 BOLTS", NOVEMBER 13, 1985
- AMERICAN WELDING SOCIETY D1.1-92. "STRUCTURAL WELDING CODE -STEEL"
- AMERICAN WELDING SOCIETY D1.3-89, "STRUCTURAL WELDING CODE -SHEET STEEL"
- AMERICAN WELDING SOCIETY D1.4-92, "STRUCTURAL WELDING CODE-REINFORCING STEEL"
- AMERICAN WELDING SOCIETY A2.4-86, "SYMBOLS FOR WELDING AND NONDESTRUCTIVE TESTING"  $\,$
- AMERICAN IRON AND STEEL INSTITUTE, "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS," AUGUST 19, 1986
- AMERICAN SOCIETY OF TESTING AND MATERIALS

MIXING AND PLACING OF ALL CONCRETE AND SELECTION OF MATERIALS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. PROPORTIONS OF AGGREGATE TO CEMENT SHALL BE SUCH AS TO PRODUCE A DENSE, WORKABLE MIX WHICH CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER. ALL CONCRETE, INCLUDING SLABS ON GROUND, SHALL HAVE AN ACCEPTABLE WATER-REDUCING ADMIXTURE ADDED IN ACCORDANCE WITH MANUFACTURER'S DIRECTIONS. IN ADDITION, ALL CONCRETE EXPOSED TO THE WEATHER SHALL CONTAIN AN ACCEPTABLE ADMIXTURE TO PRODUCE ENTRAINED AIR IN ACCORDANCE WITH TABLE 3 OF ASTM C94 FOR MODERATE EXPOSURE. TESTS FOR AIR CONTENT SHALL BE MADE AT THE DISCHARGE OF THE TRUCK IN ACCORDANCE WITH ASTM C172 REQUIREMENTS.

MAXIMUM SIZE OF AGGREGATE SHALL BE 1-1/2 INCHES, BUT MAXIMUM SIZE OF AGGREGATE SHALL NOT BE MORE THAN THREE QUARTERS OF THE CLEAR DISTANCE BETWEEN REINFORCING BARS. MAXIMUM SIZE OF AGGREGATE FOR SLABS ON GROUND SHALL BE ONE-THIRD THE THICKNESS OF THE SLAB.

MIX DESIGNS SHALL BE SUBMITTED TO THE ARCHITECT FOR ACCEPTANCE PRIOR TO USE. MAXIMUM WATER-CEMENT RATIO AND SLUMP SHALL BE AS FOLLOWS FOR VARIOUS CONCRETE STRENGTHS (f'c) BASED ON STANDARD 28-DAY CYLINDER TESTS WHEN STRENGTH DATA FROM TRIAL BATCHES OR FIELD EXPERIENCE ARE NOT AVAILABLE

## WATER-CEMENT RATIO BY WEIGHT

f'c	ENTRAINED	AIR-ENTRAINED	SLUMP	LOCATION
4,000 psi	0.44	0.40	4	ALL CONCRETE UNLES NOTED OTHERWISE
5,000 psi	0.40	0.40	4	FIRST FLOOR SLAB A CONCRETE EXPOSED TO EXTERIOR ENVIRONMEN

CONCRETE FORMWORK SHALL HAVE BUILT-IN CAMBER TO COMPENSATE FOR FORM SAG UNDER WET CONCRETE LOAD. IN ADDITION TO STRUCTURAL CAMBER NOTED. ONE-WAY SLABS SHALL BE CAMBERD 1/1000 OF THE SPAN, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM AND JOIST STRUCTURAL CAMBERS ARE NOTED ON THE DRAWINGS. CAMBERS OF LESS THAN 1/8 INCH MAY BE NEGLECTED.

ALL CONSTRUCTION JOINTS IN WALLS SHALL BE KEYED IN ACCORDANCE WITH THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS OR, AT THE CONTRACTOR'S OPTION, SHALL BE INTENTIONALLY ROUGHENED IN ACCORDANCE WITH THE FOLLOWING: THE SURFACE OF ROUGHENED JOINTS SHALL BE SAND BLASTED OR ROUGHENED WITH A CHIPPING HAMMER TO EXPOSE THE AGGREGATE EMBEDDED IN THE PREVIOUS POUR. THE EXPOSED AGGREGATE SHALL PROTRUDE A MINIMUM OF 1/4 INCH. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. INMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL BE HELD TO A MAXIMUM SPACING OF 40'-0". ALL CONSTRUCTION JOINTS IN SLABS, JOISTS, BEAMS, AND GIRDERS SHALL BE OFFSET A DISTANCE EQUAL TO TWICE THE WIDTH OF THE BEAM. THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE ENGINEER FOR ACCEPTANCE BEFORE STARTING CONSTRUCTION.

ALL CONSTRUCTION JOINTS FOR SLABS ON DECK SHALL BE IN ACCORDANCE WITH THE TYPICAL SLAB ON DECK CONSTRUCTION JOINT DETAIL SHOWN ON THE STRUCTURAL DRAWINGS.

EXCEPT AS DETAILED ON STRUCTURAL DRAWINGS, NO JOISTS, BEAMS, OR GIRDERS SHALL BE SLEEVED FOR PIPING OR DUCTS, UNLESS APPROVED BY THE ENGINEER. ELECTRICAL CONDUIT:

IF CONDUIT IS TO RUN IN SLABS, THE CONDUIT SHALL BE RUN AT MID-DEPTH OF SLAB AND CONDUIT SIZE SHALL NOT EXCEED 33 PERCENT OF THE SLAB DEPTH. NO CONDUIT SHALL BE PLACED IN SLABS WITH ACTUAL CONCRETE THICKNESS LESS THAN 3 INCHES, NOT INCLUDING METAL DECK DEPTH. THERE SHALL BE A MINIMUM OF 3 INCHES OF CLEAR SPACE BETWEEN CONDUITS. ALUMINUM CONDUIT IS PROHIBITED. PLACE \*3 AT 12 INCHES ADDITIONAL REINFORCING ABOVE AND BELOW, PERPENDICULAR TO THE CONDUIT. THE ADDED REINFORCING SHALL EXTEND 1'-O" PAST THE CONDUITS ON BOTH SIDES.

ALL REINFORCING SHALL BE NEW BILLET STOCK ASTM A 615, GRADE 60. BARS SHALL BE SECURELY TIED IN PLACE WITH \*16 DOUBLE-ANNEALED IRON WIRE. BARS SHALL BE SUPPORTED ON ACCEPTABLE CHAIRS. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES." CONTRACTOR SHALL COORDINATE REINFORCING STEEL PLACEMENT DETAILS AND PROVIDE TEMPLATES FOR PLACING STEEL IN CONGESTED AREAS AS NECESSARY. SHOP DRAWINGS (INCLUDING PLACING PLANS AND ELEVATIONS) SHALL BE SUBMITTED TO AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE STARTING FABRICATION.

NO REINFORCING BARS SHALL BE WELDED UNLESS SPECIFICALLY DETAILED.

THREADED SPLICES SHALL BE ICBO APPROVED TO ACHIEVE A TENSILE STRENGTH OF 125% OF THE BAR YIELD STRENGTH. SUBMIT TO ENGINEER FOR APPROVAL. ACCEPTABLE THREADED SPLICES SHALL BE THE LENTON REBAR SPLICE (ICBO \*3967), THE FOX-HOWLETT REBAR SPLICE (ICBO \*3085), OR THE DAYTON BAR GRIP (ICBO \*3848).

REINFORCING BARS SHALL BE LAP SPLICED FOR TENSION (LSB) UNLESS OTHERWISE NOTED ON THE DRAWINGS. NOTE: \*14 AND \*18 BARS SHALL NOT BE LAP SPLICED. WELDING OR TACK WELDING OF REINFORCING BARS TO OTHER BARS OR TO PLATES. ANGLES, ETC., IS PROHIBITED, EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. WHERE WELDING IS APPROVED, IT SHALL BE DONE BY AWS/WABO CERTIFIED WELDERS USING EPOINS OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF AWS-DI. 4.

 $\mbox{\tt MINIMUM}$  CAST-IN-PLACE CONCRETE COVER OVER REINFORCING STEEL, UNLESS OTHERWISE NOTED, SHALL BE AS FOLLOWS:

- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES
- . CONCRETE EXPOSED TO EARTH OR WEATHER: 2 INCHES . CONCRETE EXPOSED TO SEAWATER OR SEAWATER SPRAY: 2-1/2 INCHES.
- WALLS INTERIOR FACE: \*14 AND \*18 BARS 1-1/2 INCHES, \*11 BARS AND SMALLER 3/4 INCH.
  SLABS AND JOISTS: \*14 AND \*18 BARS 1-1/2 INCHES, \*11 BARS AND SMALLER BEAMS AND COLUMNS - TIES, STIRRUPS, SPIRALS: INTERIOR FRAMES - 1-1/2
  INCHES, EXTERIOR FRAMES - 2 INCHES.
  REINFORCEMENT IN COLUMNS AND WALLS THAT ARE ABOVE THE PILECAPS AND BELOW THE FIRST FLOOR SLAB SHALL BE EPOXY COATED PER REQUIREMENTS SHOWN munimum manum IN THE SPECIFICATIONS.

## PRECAST CONCRETE:

THE CONTRACTOR SHALL DESIGN, PROVIDE, AND INSTALL PRECAST CONCRETE CLADDING AND SEATING TREAD UNITS AS SHOWN IN ARCHITECTURAL AND STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS FOR OPENINGS, DETAILS, AND DIMENSIONS NOT SHOWN, HORIZONTAL PRECAST MEMBERS SHALL BE CAST, STRESSED, TRANSPORTED, AND ERECTED IN A HORIZONTAL, UPRIGH POSITION. SUPPORTS DURING TRANSPORTATION AND ERECTION SHALL APPROXIMATE THOSE IN THE FINAL STRUCTURE. WHERE NECESSARY, THE CONTRACTOR SHALL INUSE IN THE FINAL STRUCTURE. WHERE NECESSARY, THE CONTRACTOR SHALL PROVIDE AND INSTALL ADDITIONAL REINFORCING (STEEL STIFFENERS, BRACING, LIFTING INSERTS, ETC.) TO RESIST ERECTION AND TRANSPORTATION STRESSES. LIFTING INSERTS SHALL BE INSTALLED AT LOCATIONS WHERE THEY WILL BE HIDDEN BY CONSTRUCTION OR OTHERWISE COVERED. FINISH ALL PANELS WHICH WILL BE COVERED WITH TOPPING SLABS WITH ROUGH SCREED FINISH TO ENSURE BONDING OF THE CONCRETE TOPPING. SUBMIT DETAILED SHOP DRAWINGS OF ALL PRECAST CONCRETE TOPPING. SUBMIT DETAILED SHOP DRAWINGS OF ALL PRECAST CONCRETE CONNECTIONS (PLATES, ANGLES, ETC.) SHALL BE GALVANIZED STEEL. REMOVE GALVANIZING WHERE FIELD WELDING IS REQUIRED, AND PAINT FIELD WELDS WITH ONE COAT OF "GALVACON" OR APPROVED EQUAL. SEE SHEAR STUD CONNECTIONS NOTE FOR STUDS WELDED TO CONNECTIONS, FIELD WELDING SHALL BE DONE BY AWS-CERTIFIED WELDERS IN ACCORDANCE WITH AWS DI.1. SEE CURTAIN WALL INSTALLATION FOR LOADING RESTRICTIONS. NOTE BUILDING STRUCTURAL DESIGN HAS ASSUMED AN AVERAGE PRECAST PANEL THICKNESS OF 6-INCHES.

## WELDED WIRE FABRIC:

WELDED WIRE FABRIC (WWF) SHALL BE ELECTRICALLY WELDED AND CONFORM TO ASTM A185. AN 8-INCH MINIMUM LAP SHALL BE PROVIDED FOR SIDE AND END LAPS. WELDED WIRE FABRIC SHALL BE SUPPORTED ON APPROVED CHAIRS. DRILLED-IN CONCRETE ANCHORS (DICA):

ACCEPTABLE DRILLED-IN CONCRETE ANCHORS, OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE DRAWINGS, SHALL BE AS FOLLOWS: HILTI "KWIK-BOLT-II" CARBON STEEL WEDGE ANCHORS (ICBO "4627), "WEJ-IT ANCHOR BOLT" (ICBO "1821) OR "ITW RAMSET/RED HEAD TRUBOLT CARBON STEEL WEDGE ANCHORS" (ICBO \*1372). MINIMUM EMBEDMENT DEPTH SHALL BE 4.5 BOLT DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS.

## NON-SHRINK GROUT FOR BASE PLATES:

GROUT SHALL BE AN APPROVED NON-SHRINK CEMENTITIOUS GROUT CONTAINING NATURAL AGGREGATES DELIVERED TO THE JOB SITE IN FACTORY PREPACKAGED CONTAINERS REQUIRING ONLY THE ADDITION OF WATER. THE MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL BE AT LEAST 1,000 PSI HIGHER THAN THE SUPPORTING CONCRETE STRENGTH, UNLESS OTHERWISE NOTED. APPROVED GROUTS INCLUDE: MASTER BUILDERS' "MASTER FLOW 928", SIKA CORPORATION'S "SIKAGROUT 212", BURKE COMPANY'S "NON-FERROUS NON-SHRINK GROUT", OR APPROVED EQUAL. GROUT SHALL BE MIXED, APPLIED, AND CURED STRICTLY IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTRUCTIONS.

ALL STEEL SHALL CONFORM TO THE FOLLOWING:

ALL STEEL, U.N.O. ASTM A992: A572, GRADE 50: A441, 50 KSI: A588, 50 KSI MATERIAL CALLED OUT ON PLANS AS (Fy=36) ASTM A36, Fy=36 KSI ALL PLATES AND ANGLES, U.N.O. ASTM A36, Fy=36 KSI SQUARE OR RECTANGULAR STRUCTURAL TUBE ASTM A500 GRADE B, Fy=46

STEEL PIPE DIAMETER LESS THAN OR EQUAL ASTM A53, TYPE E OR S TO 12" ASTM A53, TYPE E OR S GRADE B. Fy=35 KSI

STEEL PIPE DIAMETER GREATER THAN 12" ASTM A572. GRADE 50 FABRICATED TO REQUIREMENTS

ALL WORK SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATION. SHOP DRAWINGS SHALL BE SUBMITTED AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE COMMENCING FABRICATION. ALL STEEL ANCHORS AND TIES AND OTHER MEMBERS EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED.

IMENSIONAL TOLERANCE FOR BUILT-UP MEMBERS SHALL BE PER AWS D1.1.

ASTM AG GROUP 4 AND 5 ROLLED SHAPES AND PLATES EXCEEDING 2-INCH THICKNESS USED FOR BUILT-UP MEMBERS REQUIRE SHAPPY V-NOTCH TESTING IN ACCORDANCE WITH ASTM AG SUPPLEMENTARY REQUIREMENT S5, WHERE FULL-PENETRATION WELDS ARE CALLED OUT AT EITHER END OF THE MEMBER ON THE DRAWINGS. REFER TO AISC SPECIFICATION PARAGRAPH A3.C FOR TESTING

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 7/8-INCH DIAMETER A325 BOLTS IN SINGLE SHEAR. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED AND INSPECTED IN ACCORDANCE WITH THE A1SC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. THE CRITERIA FOR SLIP-CRITICAL CONNECTIONS SHALL APPLY TO ALL CONNECTIONS UNLESS SPECIFICALLY NOTED AS SNUG-TIGHT ON THE STRUCTURAL DRAWINGS. WHERE CONNECTIONS ARE NOTED AS SNUG-TIGHT THE CONTRACTOR MAY INSTALL PER CRITERIA FOR SNUG-TIGHT BOLTS. SLIP-CRITICAL CONNECTIONS SHALL USE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. ALL ASTM A307 BOLTS SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF-LOCKING NUTS. ALL BOLT HOLES SHALL BE STANDARD SIZE, U.N.O.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO: ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS: MELDING PROCEDURES: REQUIRED ROOT OPENINGS: ROOT FACE DIMENSIONS: GROOVE ANGLES: BACKING BARS: COPES: SURFACE ROUGHNESS VALUES: AND TAPERS OF UNEQUAL PARTS,

STRUCTURAL STEEL SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY AWS CERTIFIED WELDERS AND IN ACCORDANCE WITH CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS DI.1. WELDS SHOWN ON THE DRAWINGS ARE THE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES. BASED ON PLATE THICKNESS. THE MINIMUM WELDING SHALL BE 3/16 INCH. ALL PARTIAL PENETRATION GROOVE WELD SIZES SHOWN ON THE DRAWINGS REFER TO EFFECTIVE THROAT THICKNESS.

ALL WELDS SHALL BE MADE USING LOW-HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH = 70 KSI. LOW HYDROGEN SMAW ELECTRODES SHALL BE USED WITHIN FOUR HOURS OF OPENING THEIR HERMETICALLY SEALED CONTAINERS, OR SHALL BE REDRIED PER AWS D1.1, SECTION 4.5. ELECTRODES SHALL BE REDRIED NO MORE THAN ONE TIME, AND ELECTRODES THAT HAVE BEEN WET SHALL NOT BE USED.

WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. ALL COMPLETE-PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATE LESS THAN OR EQUAL TO 1/4 INCH THICK SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS: ROOT FACE DIMENSIONS; GROOVE ANGLES; BACKING BARS: COPES; SURFACE ROUGHNESS VALUES; AND TAPERS OF UNEQUAL PARTS.

FOR (1) BRACED FRAME CONNECTIONS. (2) WELDED CONNECTIONS. AT MOMENT FRAMES BETWEEN GRIDS 16 AND 17. (3) EXHIBITION HALL TRUSS MEMBERS AND CONNECTIONS, AND (4) COMPLETE PENETRATION WELDS ON ASTM GROUP 4 AND 5 ROLLED SHAPES AND PLATES EXCEEDING 2-INCH THICKNESS USED FOR BUILT-UP MEMBERS, THE FOLLOWING ADDITIONAL REQUIREMENTS SHALL APPLY:

- ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. COPIES OF THE PS SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL
- PREHEAT SHALL BE 50 TO 75 DEGREES FAHRENHEIT ABOVE MINIMUM PREHEAT SPECIFIED BY AWS D1.1.
- 3. MAXIMUM INTERPASS TEMPERATURE SHALL BE LIMITED TO 550 DEGREES
- 4. AWS E70T-4, E70T-7, E71T-7 AND E71T-11 ELECTRODES SHALL NOT BE USED. MECHANICAL PROPERTIES OF THE IN-PLACE WELD (FILLER METAL) SHALL HAVE CHARPY V-NOTCH IMPACT TOUGHNESS OF AT LEAST 20 FOOT-POUNDS AT -20 DEGREES FAHRENHEIT.
- FIELD WELDS MAY NOT BE APPLIED OVER SHOP WELDS UNLESS A MANUFACTURER APPROVED COMPATIBLE ELECTRODE IS USED IN BOTH THE SHOP AND FIELD.
- BACKER PLATES FOR COMPLETE PENETRATION WELDS SHALL HAVE RUN-OFF TABS. RUN-OFF TABS SHALL BE REMOVED AND THE AFFECTED AREA GROUND SMOOTH AND TESTED FOR DEFECTS USING THE MAGNETIC PARTICLE METHOD.
- STEEL BACKER PLATES SHALL BE REMOVED FROM THE BOTTOM FLANGE OF BEAM-COLUMN CONNECTIONS, THE WELD SHALL BE BACK-GOUGED AND THE AREA TESTED FOR DEFECTS USING THE MAGNETIC PARTICLE METHOD. THE WELD SHALL BE COMPLETED AND REINFORCED WITH A FILLET WELD. BACK GOUGING BY AIR ARC MAY ONLY BE USED IF FOLLOWED BY GRINDING. ALTERNATIVELY, CERAMIC BACKER BARS SHAPED TO CREATE A REINFORCING FILLET MAY BE USED.

## STEEL CABLES AT EXHIBITION HALL:

STRUCTURAL STRANDS SHALL BE HELICAL STEEL WIRE STRUCTURAL STRAND CONFORMING WITH ASTM A586, CLASS A COATING THROUGHOUT. MINIMUM BREAKING STRENGTH OF STRAND SHALL BE 138 TONS. STRANDS SHALL BE PRESTRETCHED PER ASTM A586. PRESTRESS CABLES TO TO 20 TONS. BEGIN PRESTRESSING FROM EXTERIOR AND MOVE TOWARD CENTER TRUSSES. PROVIDE SHORING AS REQUIRED TO PREVENT LATERAL MOVEMENT OF TRUSSES DURING PRESTRESSING.

ANCHOR BOLTS: ANCHOR BOLTS SHALL BE ASTM A307 GRADE A STANDARD HEX HEAD FURNISHED WITH HEAVY HEX NUTS AND LOCK WASHERS. SIZES EXCEEDING A307 SHALL BE ASTM A36 ROD THREADED UNC-2A FURNISHED WITH DOUBLE HEAVY HEX NUTS, JAMMED, AT ENDS EMBEDDED IN CONCRETE. WHERE SPECIFIED ON THE DRAWINGS, ANCHOR BOLTS SHALL BE ASTM A325 FURNISHED WITH MATCHING HEAVY HEX NUTS AND LOCK WASHERS. SIZES EXCEEDING A325 SHALL BE ASTM A449 ROD THREADED UNC-2A FURNISHED WITH DOUBLE HEAVY HEX NUTS, JAMMED, AT ENDS EMBEDDED IN CONCRETE. ANCHOR BOLTS SHALL HAVE SUFFICIENT LENGTH TO PROVIDE THE MINIMUM EMBEDMENT SHOWN ON THE DRAWINGS MEASURED FROM THE FACE OF CONCRETE TO THE NEAR FACE OF THE HEAD OR NUT. ANCHOR BOLTS SHALL BE INSTALLED TO A SNUG TIGHT CONDITION. NO HEATING OR BENDING OF ANCHOR BOLTS IS PERMITTED. NO ENLARGEMENT OF ANCHOR BOLTS HOLES BY BURNING IS PERMITTED.

FLOOR BEAMS, ESPECIALLY EDGE BEAMS, TRANSFER GIRDERS, AND CANTILEVERS WILL CONTINUE TO DEFLECT WHEN ADDITIONAL LOAD IS APPLIED. THESE MEMBERS HAVE BEEN CAMBERED TO ACCOUNT FOR THE THEORETICAL DEFLECTION. HOWEVER, THIS MAY NOT OCCUR UNTIL ALL THE DEAD LOAD IS ON THE MEMBER. THE CONTRACTOR SHALL COORDINATE THE ATTACHMENT OF ANY ITEMS TO MEMBERS WHICH WILL CONTINUE TO SHORTEN OR DEFLECT DUE TO LATER STAGES OF CONSTRUCTION

## CURTAIN WALL INSTALLATION:

THE CURTAIN WALL CONNECTIONS SHALL ACCOUNT FOR CONSTRUCTION TOLERANCES, COLUMN SHORTENING, AND BEAM DEFLECTIONS. IN ADDITION, THE CURTAIN WALL DESIGN SHALL ACCOMMODATE A TYPICAL VERTICAL MOVEMENT AT EACH FLOOR OF 3/4 INCH DUE TO VARIABLE LIVE LODAING. THIS DISPLACEMENT WILL OCCUR AT THE FREE END OF CANTILEVER BEAMS AND AT MIDSPAN OF SIMPLE SPAN EDGE BEAMS. THE CURTAIN WALL SHALL ACCOMMODATE TYPICAL LATERAL MOVEMENTS OF 3/4 INCH BETWEEN ADJACENT FLOORS PERPENDICULAR AND/OR PARALIFI TO THE CURTAIN WALL.

THE EXTERIOR BEAMS HAVE BEEN DESIGNED FOR 500-POUND CONCENTRATED LOADS AT ONE FOOT ON CENTER, REPRESENTING CURTAIN WALL REACTIONS. CANTILEVERED SLABS OVER EDGE BEAMS WERE DESIGNED WITH THE SAME WALL LOAD APPLIED AT THE FREE EDGE OF THE SLAB.

THE STEEL DECK SHALL BE OF DEPTH SHOWN ON THE STRUCTURAL DRAWINGS.
GAGE OF DECK SHALL BE DETERMINED BY THE CONTRACTOR BASED ON THE SPAN
CONDITIONS, SHORING REQUIREMENTS, CONSTRUCTION LOADS, DEFLECTION
REQUIREMENTS, AND THE SUPERIMPOSED LOADS SHOWN ON THE DRAWINGS, LOAD
DIAGRAMS, AND NOTES. MINIMUM GAGE IS 20. MAXIMUM DEAD LOAD DEFLECTION IS
3/4 INCH OR L/180. WRITTEN VERIFICATION OF CONFORMANCE FOR ALL CONDITIONS
IN THE STRUCTURE SHALL BE SUBMITTED FOR ACCEPTANCE PRIOR TO
FABRICATION. THE CAPACITIES OF THE DECK SHALL BE BASED ON CURRENT ICBO
REPORTS. SHOP DRAWINGS SHALL BE SUBMITTED SHOWING DECK GAGE, LAYOUT,
FASTENING, STUD LAYOUT, AND CLOSURES. IF ANY SHORING IS TO BE USED, IT
SHALL BE APPROVED BY THE GENERAL CONTRACTOR AND SHALL BE SHOWN ON THE
SHOP DRAWINGS. UNITS SHALL SPAN OVER FOUR SUPPORTS, CONTINUOUS OVER
THREE OR MORE SPANS, EXCEPT WHERE FRAMING DOES NOT PERMIT. THE AISI
SPECIFICATIONS SHALL GOVERN THE DESIGN OF ALL DECK UNITS. STEEL DECK AND
ALL OF ITS FLASHINGS SHALL CONFORM TO ASTM A653, GRADE 33. THE
STEEL SHALL HAVE RECEIVED, BEFORE BEING FORMED, A METAL PROTECTIVE
COATING OF ZINC CONFORMING TO ASTM A 525-660. ALL WELDING SHALL BE IN
ACCORDANCE WITH AWS DI.3.

CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL. UNLESS SHOWN OTHERWISE, UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE ENDS OF THE UNITS AND AT INTERMEDIATE SUPPORTS AT 12 INCHES ON CENTER WITH 3/4-INCH-DIAMETER PUDDLE WELDS: WHERE TWO UNITS ABUT, EACH UNIT SHALL BE SO FASTENED TO THE STEEL SUPPORTS. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY 1-1/2-INCH TOP SEAM WELDS AT 2'-O" ON CENTER OR BUTTON PUNCHED AT 2'-O" ON CENTER. DECK UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4-INCH-DIAMETER PUDDLE WELDS AT 1'-O" ON CENTER. 3/4-INCH DIAMETER SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN PLACE OF 3/4-INCH DIAMETER SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN PLACE OF 3/4-INCH DIAMETER SHEAR STUDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. COORDINATE ALL CLOSURES WITH ELEVATOR, STAIR, ESCALATOR AND OTHER ARCHITECTURAL DETAILS. THE DECK INSTALLATION, WHEN COMPLETE, SHALL BE READY TO RECEIVE CONCRETE. COMPLETE, SHALL BE READY TO RECEIVE CONCRETE.

STEEL ROOF DECK:

THE STEEL DECK SHALL BE OF DEPTH AND GAGE SHOWN ON THE STRUCTURAL DRAWINGS. THE AISI SPECIFICATIONS SHALL GOVERN THE DESIGN OF ALL DECK UNITS. STEEL DECK AND ALL OF ITS FLASHINGS SHALL CONFORM TO ASTM AGE GRADE 33. THE STEEL SHALL HAVE RECEIVED, BEFORE BEING FORMED, A METAL PROTECTIVE COATING OF ZINC COMFORMING TO ASTM A 525-GGO. ALL WELDING SHALL BE IN ACCORDANCE WITH AWE DIE. SHALL BE IN ACCORDANCE WITH AWS D1.3.

NONCOMPOSITE UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE ENDS OF THE UNITS AND AT INTERMEDIATE SUPPORTS BY A TOTAL OF FOUR 3/4-INCH DIAMETER PUDDLE WELDS (1/2 INCH EFFECTIVE) PER 2'-0" OF WIDTH. WHERE TWO UNITS ABUT, EACH UNIT SHALL BE SO FASTENED TO THE STEEL FRAMING. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY 1-1/2-INCH TOP SEAM WELDING AT 2'-0" ON CENTER, UNLESS NOTED OTHERWISE. DECK UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4-INCH DIAMETER PUDDLE WELDS AT 1'-0" ON CENTER.

WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD DECK TO STEEL WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.

STEEL DECK TYPES SHALL BE UNITED STEEL TYPE NI, OR APPROVED EQUAL.

# FLOOR SLABS SHALL BE CONSTRUCTED TO THE ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR FLOOR TOLERANCES. CONTRACTOR SHALL INCLUDE THE QUANTITIES OF THE ADDED CONCRETE DUE TO THE METAL DECK DEFLECTION. DESIGN CAMBER SHOWN FOR THE STEEL BEAMS HAS BEEN CALCULATED BASED ON THE DEFLECTION OF THE BEAM DUE TO THE WEIGHT OF THE STEEL AND CONCRETE SLAB.

## FOUNDATIONS:

SOILS ENGINEER SHALL CONFIRM THAT THE TIP OF THE PILES ARE LOCATED ON PROPER BEARING MATERIAL. MASSONRY VENEER:

24" OC HORIZONTAL. 16" OC VERTICAL. PROVIDE ADDITIONAL ANCHORS AT OPENINGS LARGER THAN 16" IN EITHER DIRECTION. PLACE ANDCHORS WITHIN 12" OF OPENING.

THE STRUCTURE BY ITSELF IS A NON-SELF-SUPPORTING FRAME UNTIL ALL STRUCTURAL ELEMENTS ARE IN PLACE. LATERAL FORCES DUE TO WIND, EARTHQUAKE OR SOIL ARE CARRIED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE BRACING ELEVATIONS. CERTAIN ELEMENTS SHOWN ON THE STRUCTURAL DRAWINGS (SUCH AS BRACING, ROOF AND FLOOR SLABS, ARE REQUIRED FOR OVERALL STABILITY OF OTHER ELEMENTS (SUCH AS BEAMS, COLUMNS, TENSIONED MEMBRANE ROOF SYSTEM, AND WALLS). IF, DUE TO SEQUENCING OF CONSTRUCTION, THESE STABILITY ELEMENTS ARE NOT IN PLACE. THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER WHO SHALL INVESTIGATE WHERE TEMPORARY SHORING/BRACING IS REQUIRED, AND SHALL DESIGN THIS TEMPORARY SHORING/BRACING.

# Ce=0.7 (SNOW EXPOSURE FACTOR)

Pg=15 PSF (GROUND SNOW LOAD)

# WIND LOADS:

CURTAIN WALL CONSTRUCTION SEQUENCE SHALL BE COORDINATED BETWEEN THE CONTRACTOR, ARCHITECT, AND STRUCTURAL ENGINEER, SO THAT THE REMAINING MAGNITUDE OF DIFFERENTIAL COLUMN SHORTENING AND BEAM DEFLECTION CAN BE ESTIMATED AND PROPER TOLERANCES CAN BE BUILT INTO THE CURTAIN WALL CONNECTIONS

WALL CLADDING ATTACHMENTS SHALL NOT APPLY ANY LATERAL LOADS TO THE BOTTOM FLANGE OF THE BEAMS. IF ATTACHMENT IS MADE TO THE BOTTOM OF BEAMS, ADDITIONAL INCLINED STRUTS BRACING THE BOTTOM FLANGE OR OTHER EQUIVALENT MEANS TO COUNTERACT THIS FORCE SHALL BE PROVIDED BY THE

## SHEAR CONNECTOR STUDS:

ALL SHEAR CONNECTOR STUDS SHALL BE 3/4 INCH IN DIAMETER UNLESS OTHERWISE NOTED. ACCEPTABLE TYPES SHALL BE TRU-WELD (1080 \*3741) OR NELSON (1080 \*2614). SHEAR CONNECTOR STUDS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER OF STUDS. STEEL STUD MATERIAL, WELDING, AND INSPECTION SHALL BE IN ACCORDANCE WITH AWS D1.1. SHEAR STUDS SHALL BE PLACED AT 2'-0" ON CENTER MAXIMUM ON ALL BEAMS SUPPORTING A STEEL DECK WITH CONCRETE FILL OR A CAST-IN-PLACE CONCRETE SLAB. SEE "SHEAR STUD PLACEMENT" FOR LAYOUT CRITERIA. METAL DECK SHOP DRAWINGS DETAILING THE SHEAR STUD PLACEMENT SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE INSTALLATION.

STEEL DECK TYPES SHALL BE UNITED STEEL LOK FLOOR DECK OR APPROVED EQUAL.

MINIMUM GAGE 20: HEAVIER GAGES AS INDICATED ON THE DRAWINGS. UNITS SHALL SPAN OVER FOUR SUPPORTS (CONTINUOUS OVER THREE OR MORE SPANS), EXCEPT WHERE THE FRAMING DOES NOT PERMIT.

THE FOUNDATION DESIGN IS BASED UPON THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL REPORT ON FULL GEOTECHNICAL INVESTIGATION FOR PROPOSED NEW CONVENTION CENTER - "WILDWOOD, NEW JERSEY DATED 7-19-99" SUBMITTED BY GEOTECH, INC. THE REPORT SHALL BE MADE AVAILABLE TO THE CONTRACTOR PER PROJECT SPECIFICATIONS. REFER TO THE GEOTECHNICAL REPORT FOR THE SITE SOIL CONDITIONS.

VENEER ANCHORS TO BE "WIRE BOND CLIP" BY MASONRY REINFORCING CORP OF AMERICA OR "SEISMIC STRAP ANCHOR" BY DUR-O-WAL INC. SPACE ANCHORS AT

REFER TO ARCHITECTURAL. MECHANICAL AND ELECTRICAL PLANS FOR SIZE AND LOCATION OF CURBS. EQUIPMENT PADS. WALL AND FLOOR OPENINGS, ARCHITECTURAL TREATMENT, AND DIMENSIONS NOT SHOWN. WHERE SECTIONS ARE INDICATED ON THE PLAN BY A NUMBER AND A DRAWING NUMBER THUS, 1/S401. THE INDICATED SECTION (1) IS SHOWN ON STRUCTURAL DRAWING S401. SEQUENCING CONSTRUCTION AND LATERAL STABILITY:

STRUCTURAL DESIGN DATA:

ROOF SNOW LOAD: I=1.2 (SNOW IMPORTANCE FACTOR)

Pf=12.6 PSF (FLAT ROOF SNOW LOAD) AT EXHIBITION HALL ROOF, DESIGN SNOW LOADS VARY LINEARLY FROM 37.8 PSF (3Pf) AT THE VALLEYS TO 6.3 PSF (0.5Pf) AT THE RIDGES

BASIC WIND SPEED = 90 MPH I=1.23 (WIND LOAD IMPORTANCE FACTOR)

EXPOSURE LEVEL: D - FOR PRIMARY STRUCTURE C - FOR COMPONENTS AND CLADDING

45 FOOT MEAN HEIGHT, CLOSED STRUCTURE

WALLS: VALUES VARY WITH HEIGHT. MAXIMUM VALUES SHOWN FOR 45 FOOT MEAN HEIGHT. WINDWARD (TOWARD WALL): LEEWARD (AWAY FROM WALL)

Cp=0.5 P≈30.5 PSF

SEE DRAWING S103 FOR WIND LOADING AT TENSIONED MEMBRANE ROOF SYSTEM.

Av=0.05 (PEAK VELOCITY RELATED ACCELERATION) Ag=0.05 (PEAK ACCELERATION)

SEISMIC HAZARD EXPOSURE GROUP II SEISMIC PERFORMANCE CATAGORY B

SOIL PROFILE TYPE S2 (RESULTING IN SITE COEFFICIENT S=1.2) SEISMIC RESISTING SYSTEM (CONCENTRICALLY BRACED FRAMES) R=5, Cd=41/2 BUILDING IS DESIGNED PER EQUIVALENT LATERAL FORCE PROCEDURE AS DESCRIBED IN BOCA SECTION 1610.4.

DWG NO DRAWING TITLE GENERAL NOTES, ABBREVIATIONS & DRAWING LIST DRAWING SYMBOLS & LOAD MAPS FOUNDATION PLAN - COMPOSITI FOUNDATION PLAN - SECTOR 1 FOUNDATION PLAN - SECTOR 2 FOUNDATION PLAN - SECTOR 3 FOUNDATION PLAN - SECTOR 4 FOUNDATION PLAN - SECTOR 5

S210.1 FIRST FLOOR - COMPOSITE
S211.1 FIRST FLOOR - SECTOR 1
S211.1A FIRST FLOOR BEINFORCING PLAN - SECTOR 1
S211.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 1
S212.1 FIRST FLOOR TOP REINFORCING PLAN - SECTOR 2
S212.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 2
S212.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 2
S213.1 FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 3
S213.1B FIRST FLOOR TOP REINFORCING PLAN - SECTOR 3
S213.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 3
S214.1 FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 4
S214.1A FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 4
S214.1B FIRST FLOOR TOP REINFORCING PLAN - SECTOR 4
S215.1 FIRST FLOOR SECTOR 5
S215.1A FIRST FLOOR SECTOR 5
S215.1A FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 5
S215.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 5
S216.1 FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 5
S216.1A FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 6
S216.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 6
S216.1B FIRST FLOOR BOTTOM REINFORCING PLAN - SECTOR 6

MECHANICAL MEZZANINE - COMPOSITE MECHANICAL MEZZANINE - SECTOR 1 MECHANICAL MEZZANINE - SECTOR 2 MECHANICAL MEZZANINE - SECTOR 3 MECHANICAL MEZZANINE - SECTOR 4 MECHANICAL MEZZANINE - SECTOR 5 MECHANICAL MEZZANINE - SECTOR 6

\$230.1 \$231.1 \$232.1 \$233.1 \$234.1 \$235.1 \$236.1

S241.1 PARTIAL PLANS TYPICAL STEEL DETAILS TYPICAL CMU DETAILS

BOARDWALK EXTENSION DETAILS CONCRETE DETAILS CONCRETE DETAILS S604 STEEL DETAILS

FOR DRAWING REFERENCES NOT INCLUDED IN THIS LIST, SEE PREVIOUS PHASE DRAWINGS.

PHASE 4 DRAWING LIST

ANCHOR BOLT
AMERICAN CONCRETE INSTITUTE JOIST JOINT 1,000 POUNDS KNOCK-OUT KIPS PER SQUARE INCH N INSTITUTE OF STEEL MERICAN NATIONAL STANDARDS LINEAL FOOT LINEAL: LINEAR LONG LEGS BACK-TO-BACK LONGITUDINAL LOW POINT LONG SLOTTED (HOLES) LIGHTWEIGHT AMERICAN PLYWOOD ASSOCIATION APPROXIMATE ARCHITECTURAL; ARCHITECT ABOVE MEAN SEA LEVEL MERICAN SOCIETY FOR MATERIAL
MAXIMIM
MACHINE BOLT
MISCELLANEOUS CHANNEL
MECHANICAL
MEMBRANE
MANUF ACTURE
MINIMUM: MINUTE
MISCELLANEOUS
MASONRY OPENING CK: BLOCKING ED MOMENT FRAME BELOW MEAN SEA LEVEL STANDARD CHANNEL
CENTER TO CENTER
CENTER OF GRAVITY
CAST—IN-PLACE
CONSTRUCTION JOINT
ENTERLINE
LEARANCE: CLEAR
ONCRETE MASONRY UNIT OPPOSITE OVERSIZED (HOLES) PRECAST PRECAST CONCRETE PANEL PER<u>PE</u>NDICULAR TION: CONNECT PARTIAL PENETRATION PREFABRICATED CONTINUOUS CORRUGATED
COMPLETE PENETRATION WELD-ULTRASONIC TEST PRESTRESSED
POUNDS PER SQUARE FOOT
POUNDS PER SQUARE INCH
POST-TENSIONED
POLYVINYL CHLORIDE ERSINK: COUNTERSUNK E: REINFORCING JIAME ILL.
DIAGONAL
DIAPHRAGM
ORILLED-IN CONCRETE ANCHOR
DIMENSION
DISCONTINUED CIAL DUCTILE QUALITY EGS BACK-TO-BACK GRADE SPACING SPECIFICATIONS SOUARE SHORT SLOTTED (HOLES) ENGINEER EQUAL: EARTHQUAKE EQUIPMENT EACH SIDE ET CETERA TOP AND BOTTOM TONGUE AND GROOVE TEMPERATURE: TEMPORARY EXTEND: EXTENDED THROUGH
TOP OF CURB: TOP OF CONCRETE
TOP OF FOOTING FLAT BAR FLOOR DRAIN FOUNDATION FAR FACE FINISH FLOOR: FLOOR LINE FLANCE FIREPROOF: FULL PENETRATION
SIZE: FAR SIDE VERTICAL GRAIN WIDE FLANGE. WIDE HIGH HORIZONTAL & VERTICAL HORIZONTAL HP SHAPES; HIGH POINT HIGH STRENGTH HOLLOW STEEL SHAPE HEIGHT

801 SECOND AVENUE SUITE 501 SEATTLE WASHINGTON 98104 206.682.3460 FAX 343.9388 SOSH ARCHITECTS 1020 ATLANTIC AVENUE ATLANTIC CITY, NJ 08401 609.345.5222 FAX 345.7486 WILDWOOD NORTH
NEAR FACE
NATIONAL FOREST PRODUCTS ASSOCIATION CONTRACT SKILLING Consulting Structural and Civil Engineers Seattle, WA 98101-2699 Ph. 206/ 292-1200 Fax 206/ 292-1201 WORK POINT
WEAKENED PLANE JOINT
WEIGHT: STRUCTURAL TEE CUT
FROM W SHAPE
WELDED WIRE FABRIC

RECEIVED

OCT 2 0 2000

SYKES, O'CONNOR

SALERNO, HAZAVEH

**ABBREVIATIONS** 

CONVENTION WILDWOOD, NEW JERSEY

LMNARCHITECTS

HBGHUSSOH BABKSHIRE

PHASE 4 CONSTRUCTION DOCUMENT ISSUE 11 FEBRUARY, 2000

03-22-00 6Y DESCRIPTION 1 10-19-00 KED CONFORM DWGS -----. .....

TITI C GENERAL NOTES. **ABBREVIATIONS** 

& DRAWING LIST

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S101.1

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